



Elmore County **Elmore County PS2 Geotechnical Investigation Report**

September 3, 2026



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1. Introduction

WSP USA Inc. (WSP) presents this Geotechnical Report (Report) to Director James Roddin at Elmore County (Elmore County) for the geotechnical engineering services for the GGLO – Elmore County PS2 project located at 2180 American Legion Blvd, Mountain Home, Idaho (the Site). The Site location is presented in Figure 1. This Report presents the results of WSP’s geotechnical field exploration program and our geotechnical design recommendations for the Site.

1.1 Project Understanding

WSP understands that Elmore County is requesting geotechnical support for the GGLO – Elmore County PS2 project. Based on recent conversations with James Roddin at Elmore County, we understand that the proposed development at the Site will include a single-story mixed-use office building of approximately 9,990 square-feet, together with associated parking and drainage improvements. A Site location image is provided below.

Figure 1.0 Site Location Image



Source: Google Earth Imagery



1.2 Site Description

The Site is bounded to the north by Elmore County EMS/EAS, to the east by Veterans United Home Loans of Mountain Home, to the south by American Legion Blvd, and to the west by Elmore County Recorder. Access to the Site is obtained by a roadway used by Elmore County EMS/EAS. The ground surface at the Site is generally level and contains scattered construction debris associated with the demolition of the previous building.

1.3 Use of This Report

This Report pertains to the proposed GGLO – Elmore County PS2 project. The geotechnical recommendations provided herein are based on our understanding of the Project. If the design or layout of the proposed improvements is modified, WSP should be provided with an updated set of plans for review. Depending on the nature of the design modifications, the recommendations of this report may need to be updated to reflect the modified design. Appendix E has further information regarding the proper use and interpretation of this geotechnical report.

1.4 Objective and Scope

The objective of this geotechnical investigation is to provide geotechnical recommendations to support the design and construction of the proposed Project. WSP explored, sampled, tested, and inferred the subsurface conditions at points within the proposed Project area. A summary of the work undertaken is presented below:

- **Field Exploration:** Advance 3 borings up to 30 feet deep and conduct a small-scale pilot infiltration test to evaluate site-specific infiltration characteristics relevant to stormwater management and design.
- **Engineering Analyses:** Perform geotechnical analyses to develop design recommendations for the proposed improvements. Analyses include evaluation of bearing capacity and settlement characteristics, as well as earthwork, fill placement, and compaction recommendations applicable to site grading and construction.
- **Report:** Prepare this report which includes: a summary of the topography and surface conditions, a summary of the general geologic conditions and subsurface conditions, and geotechnical design recommendations.

2. Geotechnical Field Investigation

WSP completed a geotechnical field investigation for the Site August 10, 2026. The field investigation consisted of advancing three borings using a hollow stem auger and excavating one test pit for conducting a small-scale infiltration test. The preliminary boring and test pit locations were marked by WSP and then cleared of underground utilities by OneCall (811). Following utility clearance and consideration of site constraints, selected locations were advanced. The completed boring and test pit locations are shown below in Figure 2.

Figure 2.0 Boring and Test Pit Plan



Source: Google Earth Imagery

2.1 August 2026 Field Investigation

Three borings, BH-01, BH-03, and BH-05, were advanced on August 10, 2026, by 2R Drilling, under subcontract to WSP and under the supervision of WSP engineer Lucas Arsenith. The borings were advanced by a Diedrich D120 truck mounted drill rig and drilled using hollow stem auger methods to 17-ft, 21.5-ft, and 29.5-ft below ground surface (bgs), respectively. Appendix A contains the logs for the soil borings. The logs show the earth materials encountered, N-values, groundwater depth at the time of drilling, and the locations of samples. The logs also show the boring number, drilling date, drilling method, and the name of the WSP geotechnical engineer who logged the boring.



Soil samples were generally obtained in 18-inch intervals down to 10-feet then 5-foot intervals to 30 feet using a Standard Penetration Test (SPT) split spoon sampler with a 2-inch outer diameter and 1.4-inch inner diameter. Soil collected inside the SPT samplers was visually classified in the field and in general accordance with ASTM D2488 and D2487, placed in sealed plastic bags or containers, and stored for future reference and laboratory testing. The samplers were driven a total of 18 inches into the soil at the bottom of the boring (or until refusal, where refusal was taken as 50 blows for 6 inches or less).

The SPT samplers were driven into the soil using an automatic 140-pound hammer free-falling a vertical distance of 30 inches. The number of hammer blows for each 6 inches of penetration was recorded. The standard penetration resistance (N) is calculated as the sum of the number of blows required for the final 12 inches of sampler penetration, and the uncorrected N-value is presented in the borehole records. The SPT sampling procedures employed in the field were consistent with those described in ASTM D1586.

2R Drilling, under subcontract to WSP and under the supervision of WSP engineer Lucas Arsenith, excavated one test pit to a total depth of 5 feet bgs. The excavation was completed using a John Deere BG 50G mini excavator. A Small-Scale Pilot Infiltration Test (PIT) was conducted in the test pit at an excavation depth of 5 feet bgs. The test pit log is presented in Appendix A and a detailed description of the PIT and procedures followed in the field are described in Section 2.4.

2.2 Existing Geotechnical Data

WSP also reviewed existing and publicly available geotechnical data near the Site. Four well logs were advanced near the Site and include Well ID 307566, 307566 (1) (adjacent well), 487806, and 307353. These were advanced to 25', 25', 200', and 490' respectively. These well logs are presented in Appendix B and are primarily sand and silt mixtures from 0 feet to 10 feet bgs and then a sandy layer lying on top of basalt at approximately 12-20 feet and then basalt.

2.3 Laboratory Testing

Representative samples were selected for testing. Samples were tested to calibrate field classifications and characterize engineering and index properties of the Site soils. Laboratory testing consisted of fines content (ASTM D1140) and particle-size distribution of soils using sieve analysis (ASTM D6913). Results of the geotechnical laboratory testing for this study are presented in Appendix C. A summary of the tests performed, results, and soil type based on the Unified Soil Classification System (USCS; ASTM D2487) is provided in Table 1.



Table 1.0 Summary of Laboratory Testing for Borings and Test Pit

Sample Source / ID	Sample Depth (feet)	Soil Classification (USCS)	In Situ Moisture	Percent Gravel (%)	Percent Sand (%)	Passing No. 200 (%)
TP-01 / S2	4.5 – 5.0	ML	28.7	-	15	85
BH-01 / S5	3.0 – 4.5	ML	23.7	5	35	60
BH-03 / S12	5.5 – 7.0	SP-SM	6.8	19	69	12
BH-05 / S18	5.0 – 6.5	ML	21.4	-	25	75

2.4 Pilot Infiltration Test

A Small-Scale Pilot Infiltration Test (PIT) was performed in general accordance with Section 8200 of the ACHD Policy Manual (ACHD, 2026). The general procedure for conducting the PIT was as follows:

- Excavated the test pit with approximate area of 16 square feet to a depth of 5 feet bgs. The exact test pit dimensions are presented in the test pit log (Appendix A).
- Added water to the test pit until the depth of water was 12 inches.
- Performed the “pre-soak period” for six hours. At least 12” of water was maintained in the PIT during the pre-soak period.

A falling-head test was not feasible at this location because water did not infiltrate into the surrounding soil after the pre-soak period during the constant head test.

Table 2.0 Hydraulic Conductivity Values During Pre-Soak Period

Calculated Hydraulic Conductivity	Method Used
0.67 in / hr	Massmann et. al (2003)
2.62 in / hr	Massmann (2008) (as summarized in WSDOE Stormwater Management in Eastern Washington, August 2019)
0.078 in / hr	Allen (2017) (as summarized in WSDOT HRM, April 2019)

WSP observed an infiltration rate of nearly 2.3 in/hr at the start of the pilot infiltration test. However, after 6 hours of presoaking and soil saturation, the infiltration rate decreased to around 0.13 in/hr.

Based on field observations, design calculations, and professional engineering judgement, WSP recommends a design infiltration rate of **0.5 in/hr**. Infiltration test results and supporting graphs are presented in Appendix D.

3. Subsurface Conditions

3.1 Regional Geologic Setting

The Project area is located within the western Snake River Plain of southwestern Idaho. According to the Idaho Geological Survey (IGS 2025), the area is underlain by Pleistocene- to Pliocene-aged olivine tholeiite basalt associated with the Snake River Plain. Surficial deposits within the vicinity of the Project include alluvial sediments consisting of gravel, sand, and silt, which locally overlie basalt bedrock.

Regional soils generally consist of silty and sandy materials derived from alluvial, eolian, and basalt-derived parent materials. Variations in soil composition and thickness occur across the area as a result of depositional processes, erosion, and weathering of the underlying basalt.

3.2 Site Geology and Soil Conditions

The Geologic Map of Idaho (IGS 2025) maps the Project area as being underlain by basalt bedrock with localized surficial alluvial deposits consisting of gravel, sand, and silt. Given the inherent vertical and lateral variability of alluvial deposits, subsurface conditions and soil layer thicknesses may vary across the Project area from those encountered during the subsurface investigation.

The subsurface conditions encountered during WSP's investigation are generally consistent with the mapped geology and available geotechnical information in the vicinity of the Site. Borings BH-01, BH-03, and BH-05 generally encountered the following stratigraphic sequence:

- 0 to 1.5 feet: medium dense silty gravel (fill)
- 1.5 to 3 feet: stiff, slightly moist silt
- 3 to 4.5 feet: stiff, slightly moist sandy silt
- 4.5 to 6 feet: medium dense, slightly moist silty sand
- 6 to 15 feet: very stiff, slightly moist silt with trace sands
- 15 to 21 feet: very dense sand with silt
- 21 to 29.5 feet: hard silt with sand and weathered basalt

3.3 Groundwater

Groundwater was encountered in the borings at depths of approximately 15 feet, 15.5 feet, and 20 feet bgs in BH-01, BH-03, and BH-05, respectively. For this site and during this time of year, it is assumed that the water is generally between 15 to 20 feet below ground surface. Groundwater depths represent short-term observations that may not accurately represent long-term conditions. Groundwater levels



and/or conditions are subject to fluctuations from seasonal variations in rainfall, snowmelt, runoff, and other site-specific conditions which could result in higher or lower values than those observed during the time of this exploration.

4. Geotechnical Design Recommendations

4.1 Bearing Capacity

Shallow footings should bear directly on improved ground or structural fill overlying improved ground and compacted to at least 95% of the ASTM D1557 maximum density. For preliminary design purposes, the structural engineer can use the following foundation recommendations:

- Footings should be embedded a minimum of 2 feet below the lowest adjacent finished grade.
- Recommended maximum allowable bearing pressures are provided in Table 3.0.

Table 3.0 Design Bearing Capacity Values

Location	Bearing Capacity
Perimeter of Building	3,300 psf
Center of Building	3,400 psf

4.2 Differential Settlement

Settlement of the proposed foundations was evaluated using the methods of Janbu et al. (1956) and Schmertmann et al. (1978). Based on the design foundation loads, the calculated differential settlement between the southwest and northeast corners of the building is approximately 0.15 inches over a distance of approximately 150 feet. The estimated differential settlement is well below tolerance levels associated with this type of Project.

4.3 Earthwork and Compaction Recommendations

Geotechnical-related site construction will consist of typical site preparation, ground improvement, excavation, subgrade preparation, placement of foundations, placement and compaction of fills, and installation of utilities and underground vaults. This section discusses selected elements of these construction issues.

4.3.1 Site Preparation

Site preparation will include removal of trees and vegetation and stripping of topsoil from areas designated for fill placement, roadways, pavements, and building pads. All trees, vegetation, and deleterious materials shall be grubbed and removed from the Site.



Based on our explorations at the Site, it is anticipated that topsoil stripping depths will range from 6 to 12 inches with some localized areas that may extend to 18 inches.

Stripping and clearing limits should extend at least 5 feet beyond all building and pavement areas. Additional stripping and over-excavation may be required in localized areas if loose or excessively organic-rich surficial soils are encountered. If excessive disturbance of subgrade soil occurs during clearing and stripping, removal of the disturbed soil may be required. If space allows, the topsoil should be stockpiled for later use as landscaping fill or exported from the Site. This material should not be used as structural fill material due to the high organic content.

Localized areas of unsuitable soil (disturbed soil, minor organic debris) should be anticipated during site preparation. Unsuitable soil and debris should be removed from beneath structures and pavements and other areas planned for development. Depending on the nature of the unsuitable soil, it may be reusable as a structural fill with appropriate placement and compaction.

4.3.2 Subgrade and Foundation Preparation

WSP expects that footings will be installed directly on top of the improved ground or on a compacted structural fill leveling pad with a minimum thickness of 6 inches. The structural fill leveling pad should be compacted to a minimum of 95% of its modified Proctor (ASTM D1557) maximum dry density. Any localized zones of loose granular soil observed within the subgrade should be compacted to a density appropriate for planned development. Any organic, soft, or pumping soil observed within a subgrade should be over-excavated and replaced with suitable structural fill material. Unsuitable excavated materials should not be mixed with materials to be used as structural fill.

The subgrade could become softened and disturbed under the influence of surface water, foot traffic, and the contractor's equipment. The contractor may need to implement suitable procedures to protect the subgrade, such as excavating with a hoe without tracking on the subgrade, use of crushed rock or gravel working mat, soil admixing the subgrade, or other procedures. Subgrade that becomes softened or disturbed by the construction should be over-excavated and replaced. Footing excavations should be free of any soft or loose material and water prior to placement of reinforcing and concrete.

WSP recommends a qualified geotechnical engineer evaluate subgrade prior to construction of footing forms.

4.3.3 Shrink and Swell Percentages

Based on the laboratory gradation, the soil within the proposed excavation consists of approximately 5 percent gravel, 35 percent sand, and 60 percent material passing the No. 200 sieve and is classified as sandy silt (ML). Based on the guidance provided in FHWA PDDM Section 9.5.1, Exhibit 5.1A, an estimated loose swell factor of approximately 26 percent was developed through interpolation of the published values for sand and silt based on their relative proportions within the site soils. Using this same interpolation for the shrink factor, an approximate value of 15 percent was calculated. These values are considered an estimate for earthwork quantity evaluation.



4.3.4 Structural Fill

The term "structural fill" refers to any materials used for roadway embankments and for support beneath foundations, sidewalks, and pavements. Backfill placed in footing excavations and utility trenches is also considered structural fill. WSP's conclusions and recommendations concerning structural fill are presented in the following paragraphs.

Materials:

Structural fill should be free of organic and inorganic debris, be near the optimum moisture content, and capable of being compacted to the required specifications for application. Soils used for structural fill should not contain any significant quantities of organic matter or debris or any individual particles greater than about 6 inches in diameter. Typical structural fill materials include clean sand and gravel; well-graded mixtures of sand and gravel (commonly called "gravel borrow" or "pit-run"); mixtures of silt, sand, and gravel; crushed rock; quarry spalls; and controlled-density fill (CDF). If the onsite soils do not meet the above criteria, or cannot be reworked to a suitable condition, we recommend using imported granular fill consisting of imported, clean, well-graded sand and gravel, so long as the percent passing the U.S. No. 200 sieve is no greater than 12 percent. Other fill materials may be used with approval of the Engineer.

Fill Placement:

Fill should be placed in horizontal lifts not exceeding 12 inches in loose thickness, and each lift should be thoroughly compacted with a mechanical compactor. Any structural fill placed beneath footing locations should extend laterally outside of the footing base at a 1H:1V slope projected down and away from the bottom footing edge.

Compaction Criteria:

Using the Modified Proctor test (ASTM D1557) as a standard, we recommend that structural fill used for onsite applications be compacted to the following minimum densities.

Table 4.0 Compaction Criteria

Application	Minimum Compaction
Building Pad	95%
Footing Subgrade or bearing pad	
Sides of Footings and Stem Walls	90%
Utility trench backfill (0 to 2 feet)	95%
Utility trench backfill (2 feet and deeper)	90%
Landscaped Areas	85%

Subgrade Verification and Compaction Testing:

All structural fill should be placed over a firm subgrade prepared in accordance with the recommendations in this report. The condition of all subgrades should be verified by the geotechnical engineer before filling or construction begins. Except in landscape areas, fill soil compaction should be verified through in-place density testing and visual observation during fill placement to evaluate compaction as earthwork progresses.

Foundation subgrades should be maintained in a well-compacted state and protected from degradation prior to paving or placing of concrete. Protection measures may include restricted traffic, perimeter drain ditches, or placement of a protective gravel layer on the subgrade. Disturbed or wet areas should be removed and replaced with a suitably compacted structural fill material.

4.3.5 Wet Season Construction

Risks of wet season construction include:

- **Degradation of Exposed Subgrades:** Subgrades exposed to rain and ponded water can degrade (e.g., have reduced bearing capacity) under traffic loads such as construction trucks and equipment. The amount of degradation is greatest for soils with higher silt content. Subgrade degradation will be evidenced by “pumping” and excessive mud and silt. In some cases, it may be possible to repair the subgrade in dryer weather by re-compaction after the soil has dried out. Alternatively, soil drying admixtures could be evaluated for use on the Site.
- **Excavated Soil Becoming Too Wet:** If soil stockpiles are not covered and protected from external moisture sources, the soil could become too wet to reuse, compact, or handle. If the soil becomes too wet, it may need to be dried out and reworked during dryer and warmer weather. The extent of impact on soils will vary— soils with higher silt content, such as till, will be most impacted by elevated moisture contents.
- **Imported Soil:** If onsite soils become too wet, and construction needs to proceed using imported fill, we recommend that the imported material contain less than 12 percent fines by weight passing the U.S. No. 200 sieve.

4.3.6 Temporary Slopes

Safe temporary slopes are the responsibility of the contractor and should comply with all applicable Occupational Safety and Health Administration (OSHA) requirements, including Section 1926 Subpart P, Appendix B. Based on the guidance presented in OSHA Appendix B, temporary excavations in the native soils may generally be constructed with stable slopes up to 6 feet in height.

If groundwater seepage is encountered during excavation, the contractor may need to install temporary drainage measures to protect the cut face and prevent degradation of the excavation area until permanent drainage measures can be constructed.

5. Additional Services

WSP is prepared to review the Project's construction documents before they are finalized. This review is necessary to verify that the geotechnical recommendations contained in this report have been properly interpreted and implemented into the Project's design. If WSP does not perform this review, then WSP can assume no responsibility for misinterpretation of the geotechnical recommendations provided herein.

The construction process is an integral design component with respect to the geotechnical aspects of a project. Geotechnical engineering is not an exact science because of the variability of natural processes. Only a very small portion of the subsurface materials that will affect the performance of the proposed project have been observed, sampled, and tested. Unanticipated or changed conditions can occur during grading and excavating (see Appendix E). Proper geotechnical observation and testing during construction is necessary to allow the geotechnical engineer the opportunity to verify design assumptions. While not within the current scope of services, WSP is available to provide construction observation and inspection services as well as providing design recommendations if subsurface conditions or methods of construction differ from those assumed in this report.

6. Limitations

This report has been prepared exclusively for the use of Elmore County and their consultants and contractors for specific application for the GGLO – Elmore County PS2 Improvements located at 2180 American Legion Blvd, Mountain Home, Idaho.

We encourage review of this report by bidders and/or contractors as it relates to factual data only (exploration records, laboratory test results). The findings, conclusions, and recommendations presented in this report were prepared in a manner consistent with the level of care and skill ordinarily exercised by other members of the geotechnical engineering profession currently practicing under similar conditions subject to the time limits and financial, physical, and other constraints applicable to the scope of work. No warranty, express or implied, is made. Appendix E contains further information regarding the proper use and interpretation of this geotechnical report.

Subsurface explorations and associated laboratory testing describe subsurface conditions only at the sites of subsurface exploration and at the intervals where samples were collected from. The data was interpreted by WSP staff, who then applied a professional geotechnical opinion regarding general subsurface conditions. The distribution, continuity, thickness, and characteristics of identified subsurface materials may vary considerably from the indicated subsurface data.

This report contains information that may be useful in the preparation of contract specifications and contractor cost estimates. However, this report is not written as a specification document and may not contain sufficient information for this use without proper modification.

7. Closing

We appreciate this opportunity to support Elmore County on this Project. Please contact Dustin Woods, PE at dustin.woods@wsp.com or (208) 514-5251 with any questions about the contents of this report.

WSP USA Inc.

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Signature:



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Appendix A Exploration Logs

RECORD OF BOREHOLE: BH-01

Sheet 1 of 1

CLIENT:Elmore County

PROJECT:US-Elmore County PS2 Support

PROJECT NO:US0057480.0861

LOCATION:Mountain Home, ID

DATE:August 10, 2026

CONTRACTOR:2R Drilling

ELEVATION:3144.0 ft (Ground)

COORDINATES:Lat: 43.134223° Long: -115.673226°

COORD SYS:SP ID West FIPS 1103 Ft

HORZ DATUM:NAD83

HOLE LOC:Southwest corner of proposed building

DEPTH (ft)	DRILL RIG	DRILL METHOD	MATERIAL PROFILE				SAMPLES				WATER CONTENT					GRAVEL (%)	SAND (%)	FINES (%)	ADDITIONAL LAB TESTING	SPT N Value				GROUNDWATER OBSERVATIONS			
			DESCRIPTION	USCS	STRATA PLOT	ELEV. DEPTH (ft)	Hammer: Automatic, 140lb, 30" drop ASTM D1586, Blows per 6 in 140-lb hammer, 30-in drop				Plastic & Liquid Limits (%)									PENETRATION RESISTANCE BLOWS/FT							
							NUMBER	TYPE	REC %	BLOWS	N-VALUE	NP	Water Content (%)	Nonplastic													
1	Diedrich D120 Truck Rig Hollow Stem Auger		Medium dense, brown, dry, Silty GRAVEL, some cobbles (GM), medium sand, fine to coarse gravel. (FILL)	GM		0.0	S3	SPT	33	4-3-3	6																
2			Stiff, brown, slightly moist, SILT (ML), medium pasticity.			1.5	S4	SPT	100	4-5-7	12																
3			Stiff, brown, slightly moist, Sandy SILT (ML), medium pasticity.	ML		3.0	S5	SPT	100	3-4-5	9																
4						4.5	S6	SPT	75	6-7-10	17																
5			Medium dense, tan to brown, slightly moist, Silty SAND (SM).	SM		6.0	S7	SPT	93	7-8-17	25																
6			Very stiff, brown, slightly moist, SILT with trace sands (ML).	ML																							
7																											
8																											
9																											
10																											
11																											
12																											
13																											
14																											
15						15.0																					
16			Very dense, brown to black, wet, SAND with silt (SM), some gravels, weathered rock (basalt).	SM			S8	SPT	54	21-23-23	46																
17			End of hole at 17.00 ft. Groundwater encountered in sampler at 15 ft.				3127.0																				
18																											
19																											
20																											
21																											
22																											
23																											
24																											
25																											

WSP Log Imperial / Soil Gradation 2 / Imperial US v02 / ASTM D2487 Auto (recommended ASTM) / 2026-08-25

RECORD OF BOREHOLE: BH-03

Sheet 1 of 1

CLIENT:Elmore County

PROJECT:US-Elmore County PS2 Support

PROJECT NO:US0057480.0861

LOCATION:Mountain Home, ID

DATE:August 10, 2026

CONTRACTOR:2R Drilling

ELEVATION:3144.0 ft (Ground)

COORDINATES:Lat: 43.134301° Long: -115.672995°

COORD SYS:SP ID West FIPS 1103 Ft

HORZ DATUM:NAD83

HOLE LOC:Center of the proposed building

DEPTH (ft)	DRILL RIG	DRILL METHOD	MATERIAL PROFILE				SAMPLES				WATER CONTENT										GRAVEL (%)	SAND (%)	FINES (%)	ADDITIONAL LAB TESTING	SPT N Value				GROUNDWATER OBSERVATIONS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						
			DESCRIPTION	USCS	STRATA PLOT	ELEV. DEPTH (ft)	NUMBER	TYPE	REC %	BLOWS	N-VALUE	H	Plastic & Liquid Limits (%)												PENETRATION RESISTANCE BLOWS/FT																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
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1	Diedrich D120 Truck Rig Hollow Stem Auger		Medium dense, brown, dry, Silty GRAVEL (GM), some cobbles, medium sand, fine to coarse gravel. (FILL)	GM		0.0 3142.5	S9	SPT	50	9-5-4	9																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								

WSP Log Imperial / Soil Gradation 2 / Imperial US v02 / ASTM D2487 Auto (recommended ASTM) / 2026-08-25

RECORD OF BOREHOLE: BH-05

Sheet 1 of 2

CLIENT:Elmore County

PROJECT:US-Elmore County PS2 Support

PROJECT NO:US0057480.0861

LOCATION:Mountain Home, ID

DATE:August 10, 2026

CONTRACTOR:2R Drilling

ELEVATION:3144.0 ft (Ground)

COORDINATES:Lat: 43.134397° Long: -115.672749°

COORD SYS:SP ID West FIPS 1103 Ft

HORZ DATUM:NAD83

HOLE LOC:Northeast corner of proposed building

DEPTH (ft)	DRILL RIG	DRILL METHOD	MATERIAL PROFILE				SAMPLES				WATER CONTENT												GRAVEL (%)	SAND (%)	FINES (%)	ADDITIONAL LAB TESTING	SPT N Value					GROUNDWATER OBSERVATIONS																																																		
			DESCRIPTION	USCS	STRATA PLOT	ELEV. DEPTH (ft)	Hammer: Automatic, 140lb, 30" drop ASTM D1586, Blows per 6 in 140-lb hammer, 30-in drop				N-VALUE	Plastic & Liquid Limits (%) Water Content (%) NP Nonplastic																																																																						
							NUMBER	TYPE	REC %	BLOWS		20	25	50	75	100	125	150																																																																
1	Diedrich D120 Truck Rig Hollow Stem Auger		Medium stiff, brown, slightly moist, SILT (ML), medium plasticity.	ML		0.0	S16	SPT	95	3-3-4	7	0	25	75	Water content: 21.4%	▲	20	40	60	80																																																														
3142.5																																																																																		
2																																																																																		
3			Medium stiff, brown, slightly moist, SILT with trace gravel (ML).	ML		2.5	S17	SPT	95	5-5-4	9											0	25	75	Water content: 21.4%	▲	20	40	60	80																																																				
3140.0																																																																																		
4																																																																																		
5			Soft, tan to brown, moist, SILT with Sand (ML).			5.0	S18	SPT	100	0-0-5	5																					0	25	75	Water content: 21.4%	▲	20	40	60	80																																										
3137.5																																																																																		
6						6.5	S19	SPT	100	5-6-8	14																															0	25	75	Water content: 21.4%	▲	20	40	60	80																																
3136.0																																																																																		
7			Stiff, brown, slightly moist, SILT with trace sands and gravels (ML).	ML		8.0	S20	SPT	100	15-22-27	49																																									0	25	75	Water content: 21.4%	▲	20	40	60	80																						
3134.5																																																																																		
8																																																																																		
9			Hard, brown, slightly moist, SILT (ML).																																																																															
10																																																																																		
11																																																																																		
12																																																																																		
13																																																																																		
14																																																																																		
15			Dense, brown, wet, sandy SILT (SM).	SM		15.0	S21	SPT	72	10-15-20	35																																																			0	25	75	Water content: 21.4%	▲	20	40	60	80												
3127.5																																																																																		
16																																																																																		
17																																																																																		
18																																																																																		
19																																																																																		
20																																																																																		
21	Hard, brown, wet, SILT with Sand (ML), weathered rock (basalt).	ML		21.0	S22	SPT	100	17-13-43	56	0	25	75	Water content: 21.4%	▲	20	40	60	80																																																																
3121.5																																																																																		
22																																																																																		
23																																																																																		
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25																																																																																		

Continued on Next Page

HAMMER TYPE: Automatic, 140lb, 30" drop



LOGGED: Lucas Arsenith

CHECKED: Dustin Woods

DATE: Aug 10, 2026

DATE: Aug 25, 2026

REV:
0

RECORD OF BOREHOLE: BH-05

CLIENT:Elmore County

PROJECT:US-Elmore County PS2 Support

PROJECT NO:US0057480.0861

LOCATION:Mountain Home, ID

DATE:August 10, 2026

CONTRACTOR:2R Drilling

ELEVATION:3144.0 ft (Ground)

COORDINATES:Lat: 43.134397° Long: -115.672749°

COORD SYS:SP ID West FIPS 1103 Ft

HORZ DATUM:NAD83

HOLE LOC:Northeast corner of proposed building

DEPTH (ft)	DRILL RIG	DRILL METHOD	MATERIAL PROFILE				SAMPLES				WATER CONTENT										GRAVEL (%)	SAND (%)	FINES (%)	ADDITIONAL LAB TESTING	SPT N Value				GROUNDWATER OBSERVATIONS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
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RECORD OF TEST PIT: TP-01

Sheet 1 of 1

CLIENT:Elmore County

PROJECT:US-Elmore County PS2 Support

PROJECT NO:US0057480.0861

LOCATION:Mountain Home, ID

DATE:August 10, 2026

CONTRACTOR:2R Drilling

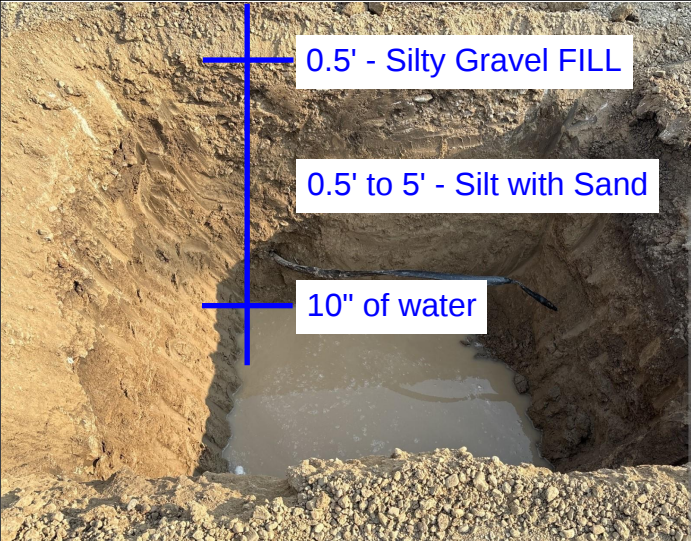
GROUND ELEV:3144.0 ft

COORDINATES:Lat: 43.134081° Long: -115.672908°

COORD SYS:SP ID West FIPS 1103 Ft

HORZ DATUM:NAD83

DEPTH (ft)	EQUIPMENT	METHOD	MATERIAL PROFILE			GRAB NUMBER	GRAB	SOIL CONSISTENCY	SOIL MOISTURE	ADDITIONAL LAB TESTING
			DESCRIPTION	USCS	STRATA PLOT	ELEV. DEPTH (ft)				
1	John Deere 50G Mini Excavator	Bucket	Medium dense, brown, dry, Silty GRAVEL (GM), some cobbles, medium sand, fine to coarse gravel. (FILL)	GM		0.0	S1			
			Soft to medium stiff, brown, slightly moist, SILT with some sands (ML).	ML		3143.5 0.5				
2			Medium stiff, brown, slightly moist, SILT with some sands (ML), medium plasticity.			3142.5 1.5				
3			(ML) Medium stiff, brown, slightly moist, SILT with trace sands, medium plasticity.			3141.0 3.0				
4			Medium stiff, brown, slightly moist, SILT with Sand (ML), medium plasticity.	ML		4.5	S2		Water content: 28.7%	
5			Final depth of 5.00 ft.			3139.0 3139.0				



Approximate PIT dimensions (L x W x D): 4' x 4' x 5'

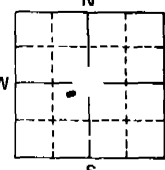
Appendix B Historical Boring Logs

RECEIVED
JUL 25 1990STATE OF IDAHO
DEPARTMENT OF WATER RESOURCESUSE TYPEWRITER OR
BALLPOINT PEN

WELL DRILLER'S REPORT

Department of Water Resources that this report be filed with the Director, Department of Water Resources
within 30 days after the completion or abandonment of the well.

JUL 23 1990

1. WELL OWNER Name <u>Earl Stone</u> mail: <u>220 N. Main</u> Address <u>American Legion Bldg, Mt. Home, Id 83647</u> Owner's Permit No. <u>61-90-2-015</u>	7. WATER LEVEL Department of Water Resources Western Regional Office Static water level <u>390</u> feet below land surface. Flowing? ☐ Yes ☒ No G.P.M. flow _____ Artesian closed-in pressure _____ p.s.i. Controlled by: ☐ Valve ☐ Cap ☐ Plug Temperature _____ °F. Quality _____ <i>Describe artesian or temperature zones below.</i>																																																																																																																																				
2. NATURE OF WORK ☐ New well ☐ Deepened ☒ Replacement ☐ Abandoned (describe abandonment procedures such as materials, plug depths, etc. in lithologic log)	8. WELL TEST DATA ☐ Pump ☐ Bailer ☒ Air ☐ Other _____ <table border="1"><thead><tr><th>Discharge G.P.M.</th><th>Pumping Level</th><th>Hours Pumped</th></tr></thead><tbody><tr><td><u>12</u></td><td></td><td><u>1</u></td></tr></tbody></table>	Discharge G.P.M.	Pumping Level	Hours Pumped	<u>12</u>		<u>1</u>																																																																																																																														
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3. PROPOSED USE ☒ Domestic ☐ Irrigation ☐ Test ☐ Municipal ☐ Industrial ☐ Stock ☐ Waste Disposal or Injection ☐ Other _____ (specify type)	9. LITHOLOGIC LOG 022414 <table border="1"><thead><tr><th rowspan="2">Bore Diam.</th><th colspan="2">Depth</th><th rowspan="2">Material</th><th rowspan="2">Water Yes No</th></tr><tr><th>From</th><th>To</th></tr></thead><tbody><tr><td>10"</td><td>0</td><td>4</td><td>Sand</td><td></td></tr><tr><td>10"</td><td>4</td><td>21</td><td>Sand & gravel</td><td></td></tr><tr><td>10"</td><td>21</td><td>32</td><td>gray lava</td><td></td></tr><tr><td>8"</td><td>32</td><td>35</td><td>Broken lava</td><td></td></tr><tr><td>8"</td><td>35</td><td>52</td><td>soft gray lava</td><td></td></tr><tr><td>8"</td><td>52</td><td>53</td><td>gray lava</td><td></td></tr><tr><td>8"</td><td>53</td><td>65</td><td>broken gray lava</td><td></td></tr><tr><td>8"</td><td>65</td><td>80</td><td>Broken gray lava</td><td></td></tr><tr><td>6"</td><td>80</td><td>90</td><td>Broken gray lava</td><td></td></tr><tr><td>6"</td><td>90</td><td>104</td><td>Brown lava</td><td></td></tr><tr><td>6"</td><td>104</td><td>109</td><td>Brown cinders</td><td></td></tr><tr><td>6"</td><td>109</td><td>145</td><td>gray lava</td><td></td></tr><tr><td>6"</td><td>145</td><td>150</td><td>Brown cinders</td><td></td></tr><tr><td>6"</td><td>150</td><td>156</td><td>gray lava</td><td></td></tr><tr><td>6"</td><td>156</td><td>189</td><td>Brown cinders</td><td></td></tr><tr><td>6"</td><td>189</td><td>230</td><td>gray lava</td><td></td></tr><tr><td>6"</td><td>230</td><td>245</td><td>Brown cinders</td><td></td></tr><tr><td>6"</td><td>245</td><td>268</td><td>gray lava</td><td></td></tr><tr><td>6"</td><td>268</td><td>280</td><td>Brown cinders & clay</td><td></td></tr><tr><td>6"</td><td>280</td><td>350</td><td>gray lava</td><td></td></tr><tr><td>6"</td><td>350</td><td>450</td><td>Brown & black cinders</td><td>X</td></tr><tr><td>6"</td><td>450</td><td>470</td><td>Brown clay & cinders</td><td>X</td></tr><tr><td>6"</td><td>470</td><td>490</td><td>Brown cinders & greenish clay</td><td>X</td></tr><tr><td>6"</td><td>490</td><td></td><td>course sand</td><td>X</td></tr><tr><td colspan="5">Hole filled in to 450'</td></tr></tbody></table>	Bore Diam.	Depth		Material	Water Yes No	From	To	10"	0	4	Sand		10"	4	21	Sand & gravel		10"	21	32	gray lava		8"	32	35	Broken lava		8"	35	52	soft gray lava		8"	52	53	gray lava		8"	53	65	broken gray lava		8"	65	80	Broken gray lava		6"	80	90	Broken gray lava		6"	90	104	Brown lava		6"	104	109	Brown cinders		6"	109	145	gray lava		6"	145	150	Brown cinders		6"	150	156	gray lava		6"	156	189	Brown cinders		6"	189	230	gray lava		6"	230	245	Brown cinders		6"	245	268	gray lava		6"	268	280	Brown cinders & clay		6"	280	350	gray lava		6"	350	450	Brown & black cinders	X	6"	450	470	Brown clay & cinders	X	6"	470	490	Brown cinders & greenish clay	X	6"	490		course sand	X	Hole filled in to 450'				
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4. METHOD DRILLED ☒ Rotary ☒ Air ☐ Hydraulic ☐ Reverse rotary ☐ Cable ☐ Dug ☐ Other _____																																																																																																																																					
5. WELL CONSTRUCTION Casing schedule: ☒ Steel ☐ Concrete ☐ Other _____ <table border="1"><thead><tr><th>Thickness</th><th>Diameter</th><th>From</th><th>To</th></tr></thead><tbody><tr><td><u>1.250</u> inches</td><td><u>8.625</u> inches</td><td><u>1</u> feet</td><td><u>24</u> feet</td></tr><tr><td><u>2.50</u> inches</td><td><u>8.625</u> inches</td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td></tr></tbody></table> Was casing drive shoe used? ☒ Yes ☐ No Was a packer or seal used? ☐ Yes ☒ No Perforated? ☐ Yes ☒ No How perforated? ☐ Factory ☐ Knife ☐ Torch Size of perforation _____ inches by _____ inches <table border="1"><thead><tr><th>Number</th><th>From</th><th>To</th></tr></thead><tbody><tr><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td></tr></tbody></table> Well screen installed? ☐ Yes ☒ No Manufacturer's name _____ Type _____ Model No. _____ Diameter _____ Slot size _____ Set from _____ feet to _____ feet Diameter _____ Slot size _____ Set from _____ feet to _____ feet Gravel packed? ☐ Yes ☒ No ☐ Size of gravel _____ Placed from _____ feet to _____ feet Surface seal depth <u>24</u> Material used in seal: ☐ Cement grout ☒ Bentonite ☐ Pudding clay ☐ _____ Sealing procedure used: ☐ Slurry pit ☐ Temp. surface casing ☒ Overbore to seal depth Method of joining casing: ☐ Threaded ☒ Welded ☐ Solvent Weld _____ ☐ Cemented between strata Describe access port _____	Thickness	Diameter	From	To	<u>1.250</u> inches	<u>8.625</u> inches	<u>1</u> feet	<u>24</u> feet	<u>2.50</u> inches	<u>8.625</u> inches											Number	From	To																																																																																																														
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6. LOCATION OF WELL Sketch map location must agree with written location.  Subdivision Name _____ Lot No. _____ Block No. _____ County <u>Elmore</u> <u>NE</u> 1/4 SW 1/4 Sec. <u>30</u> , T. <u>3</u> , N/S. R. <u>7</u> E/W	11. DRILLERS CERTIFICATION <u>DO</u> I/We certify that all minimum well construction standards were complied with at the time the rig was removed. Firm Name <u>H. M. Weston & Son, Inc.</u> Firm No. <u>35</u> Address <u>Mt. Home, Id</u> Date <u>7-19-90</u> Signed by (Firm Official) <u>Mark S. Neddolke</u> and (Operator) <u>Mark S. Neddolke</u>																																																																																																																																				
10. Work started <u>6/21/90</u> finished <u>6/22/90</u>																																																																																																																																					

WELL DRILLER'S REPORT

State law requires that this report be filed with the Director, Department of Water Resources within 30 days after the completion or abandonment of the well.

[illegible]

USE ADDITIONAL SHEETS IF NECESSARY — FORWARD THE WHITE COPY TO THE DEPARTMENT

IDAHO DEPARTMENT OF WATER RESOURCES

WELL DRILLER'S REPORT

61

1. WELL TAG NO. D 0108096
Drilling Permit No. 924361
Water right or injection well #

2. OWNER:
Name Intermountain Gas Co
Address 555 S Cole Rd
City Boise State ID Zip 83709

3. WELL LOCATION:
Twp. 3 North or South Rge. 6 East or West
Sec. 25 NE 1/4 NE 1/4
Gov't Lot County Elmore
Lat. 43° 8.26854' (Deg. and Decimal minutes)
Long. -115° 40.66065' (Deg. and Decimal minutes)

Address of Well Site E 8th N and N 18th E
(50' NW of intersection) City Mountain Home
(Give at least name of road - Distance to Road or Landmark)
Lot. Blk. Sub. Name

4. USE:
☐ Domestic ☐ Municipal ☐ Monitor ☐ Irrigation ☐ Thermal ☐ Injection
☒ Other Cathodic Protection

5. TYPE OF WORK:
☒ New well ☐ Replacement well ☐ Modify existing well
☐ Abandonment ☐ Other

6. DRILL METHOD:
☐ Air Rotary ☒ Mud Rotary ☐ Cable ☐ Other

Seal material	From (ft)	To (ft)	Quantity (lbs or ft ³)	Placement method/procedure
SC-3	200	100	2,500 lbs	Tremie Pipe
Bentonite Grout	100	0	850 lbs	Tremie Pipe

8. CASING/LINER:				
Diameter (nominal)	From (ft)	To (ft)	Gauge/Schedule	Material
1"	0	100	sch 40	PVC
1.25"	116	200	sch 40	Steel

Was drive shoe used? ☐ Y ☒ N Shoe Depth(s)

9. PERFORATIONS/SCREENS:
Perforations ☐ Y ☒ N Method

Manufactured screen ☒ Y ☐ N Type PVC All Vent

Method of installation Lowered

From (ft)	To (ft)	Slot size	Number/ft	Diameter (nominal)	Material	Gauge or Schedule
99	199	.006	100	1"	PVC	sch 40

Length of Headpipe Length of Tailpipe

Packer ☐ Y ☒ N Type

Filter Material	From (ft)	To (ft)	Quantity (lbs or ft ³)	Placement method
-----------------	-----------	---------	------------------------------------	------------------

11. FLOWING ARTESIAN:

Flowing Artesian? ☐ Y ☒ N Artesian Pressure (PSIG)

Describe control device

12. STATIC WATER LEVEL and WELL TESTS:

Depth first water encountered (ft) Static water level (ft)

Water temp. (°F) Bottom hole temp. (°F)

Describe access port

Well test:	Test method:
Drawdown (feet)	Pump ☐ Bailer ☐ Air ☐ Flowing artesian ☐
Discharge or yield (gpm)	
Test duration (minutes)	

Water quality test or comments:

13. LITHOLOGIC LOG and/or repairs or abandonment:				Water	
Bore Dia. (in)	From (ft)	To (ft)	Remarks, lithology or description of repairs or abandonment, water temp.	Y	N
12	0	6	Soft Brown Clay		X
12	6	18	Basalt Cobbles		X
8	18	20	Cinders Boulder		X
8	20	25	Voids - no circulation		X
8	25	90	Medium Hard, Broken		X
8	90	95	Medium Hard		X
8	95	108	Very Hard		X
8	108	131	Medium Hard		X
8	131	135	Soft		X
8	135	142	Medium Hard		X
8	142	145	Very Soft		X
8	145	171	Medium Hard		X
8	171	182	Very Hard		X
8	182	190	Medium Hard		X
8	190	200	Very Hard		X
			Installed 10 - 5' Anodes on 8' Centers from 118'-195'		

RECEIVED

MAY 21 2026

WATER RESOURCES
WESTERN REGION

Completed Depth (Measurable): 200'

Date Started: 05/01/2026

Date Completed: 05/06/2026

14. DRILLER'S CERTIFICATION:

I/We certify that all minimum well construction standards were complied with at the time the rig was removed.

Company Name Hansen Drilling Co. No. 694

*Principal Driller Mark Blackburn Date 05/06/2026

*Driller John Bronder Date 05/06/2026

*Operator Date

*Operator Date

Other Date

Other Date

* Signature of Principal Driller and rig operator are required.

USE ADDITIONAL SHEETS IF NECESSARY — FORWARD THE WHITE COPY TO THE DEPARTMENT

Appendix C Geotechnical Lab Results



Summary of Laboratory Test Results

Project: Elmore County PS2 (US0057480.0861)
Client: WSP USA

Project Number: B026280T
Date: 8/20/2026

Sample Source	Depth (Feet)	Lab Number	Soil Classification	In Situ Moisture, %	Passing No. 200, %	Atterberg Limits		Fines Class.
						LL	PI	
PIT-01	4.5 - 5.0	73696	Silt with Sand (ML)	28.7	85	23	1	ML
BH-01	3.0 - 4.5	73697	Sandy Silt (ML)	23.7	60	23	2	ML
BH-03	5.5 - 7.0	73698	Poorly Graded Sand with Silt and Gravel (SP-SM)	6.8	12	NV	NV	ML
BH-05	5.0 - 6.5	73699	Silt with Sand (ML)	21.4	75	22	3	ML

*NV = No Value/Non Plastic

Reviewed By:

Keith Wildman
Laboratory Services Coordinator

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ASTM D7928

Report Date: 08/19/2026

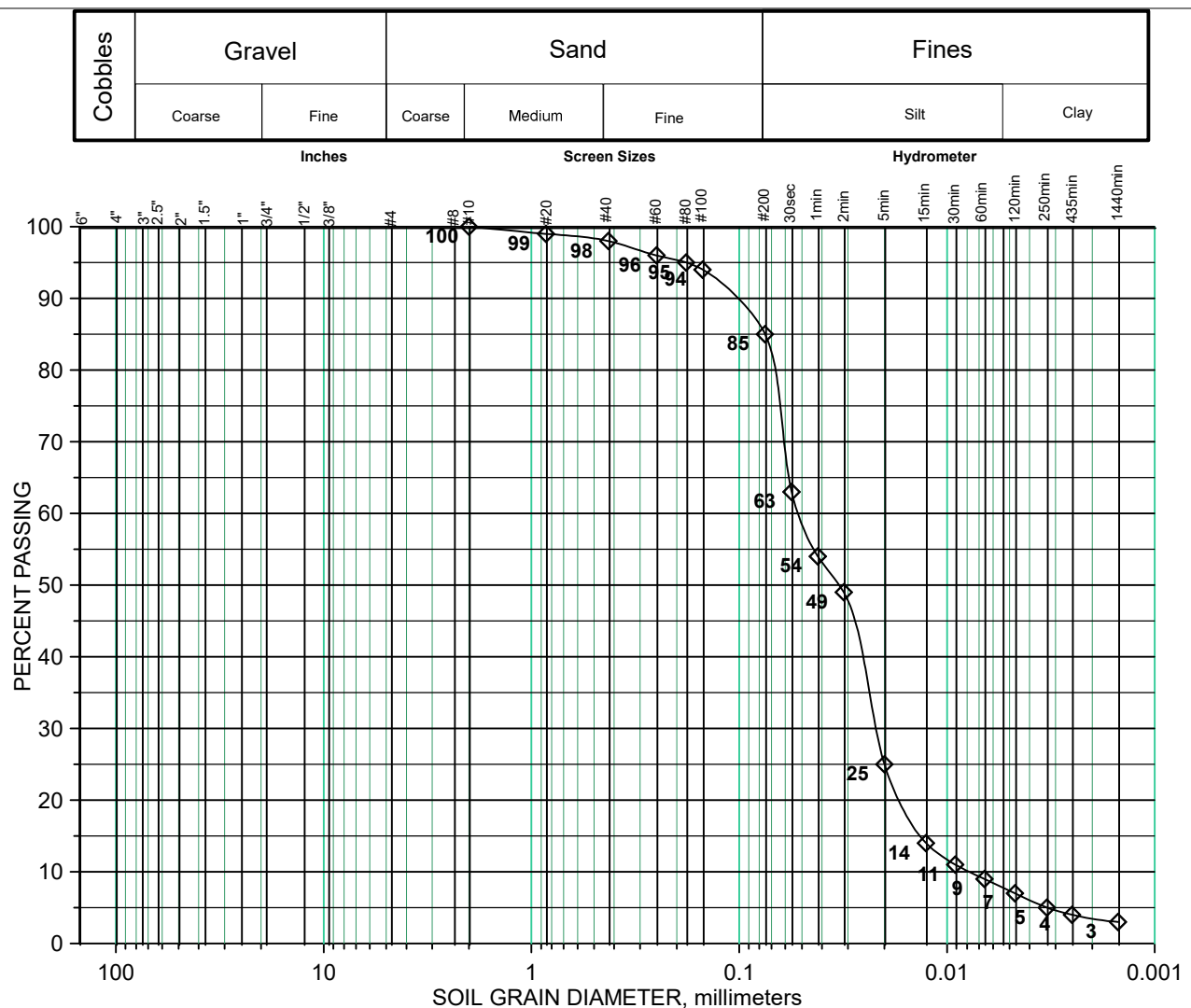
Boise
8653 West Hackamore Drive
Boise, ID 83709
Phone: 208.376.8200

Client
WSP USA
1032 S Silverstone Way
Meridian, ID 83642

Project
BO26280T
US0057480.0861
Elmore County PS2

SAMPLE INFORMATION

SAMPLE NO.:	73696	SAMPLE DATE:	08/10/2026
SPECIMEN SIZE:	Gallon baggie	TEST DATE:	08/19/2026
USCS CLASSIFICATION:	Silt with Sand (ML)	SAMPLED BY:	L. Arsenith & J. Nagle
		TESTED BY:	D. Wheeler
METHOD:	Bulk	LOCATION:	PIT-01
		DEPTH (FT):	4.5-5.0



Keith Wildman
Reviewed by Keith Wildman
Laboratory Services Coordinator

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ASTM D6913-17

Report Date: 08/20/2026

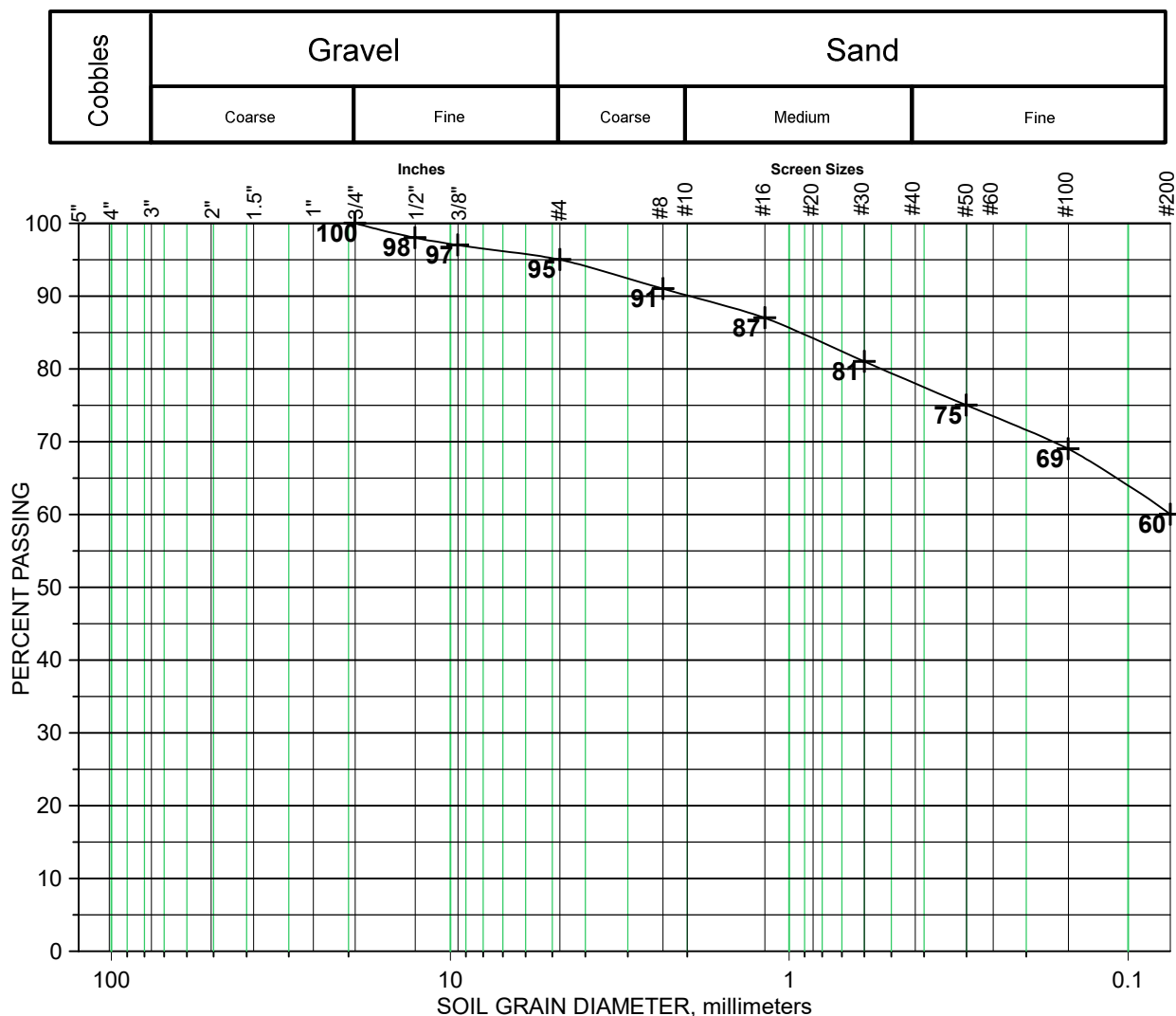
Boise
8653 West Hackamore Drive
Boise, ID 83709
Phone: 208.376.8200

Client
WSP USA
1032 S Silverstone Way
Meridian, ID 83642

Project
BO26280T
US0057480.0861
Elmore County PS2

SAMPLE INFORMATION

SAMPLE NO.:	73697	SAMPLE DATE:	08/10/2026
SPECIMEN SIZE:	Gallon bag	TEST DATE:	08/20/2026
USCS CLASSIFICATION:	Sandy Silt (ML)	SAMPLED BY:	L. Arsenith / J. Nagle
		TESTED BY:	B. Walker
METHOD:	Bulk	BORING NO.:	BH-01
		DEPTH (FT):	3.0 - 4.5



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Laboratory Services Coordinator

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ASTM D6913-17

Report Date: 08/20/2026

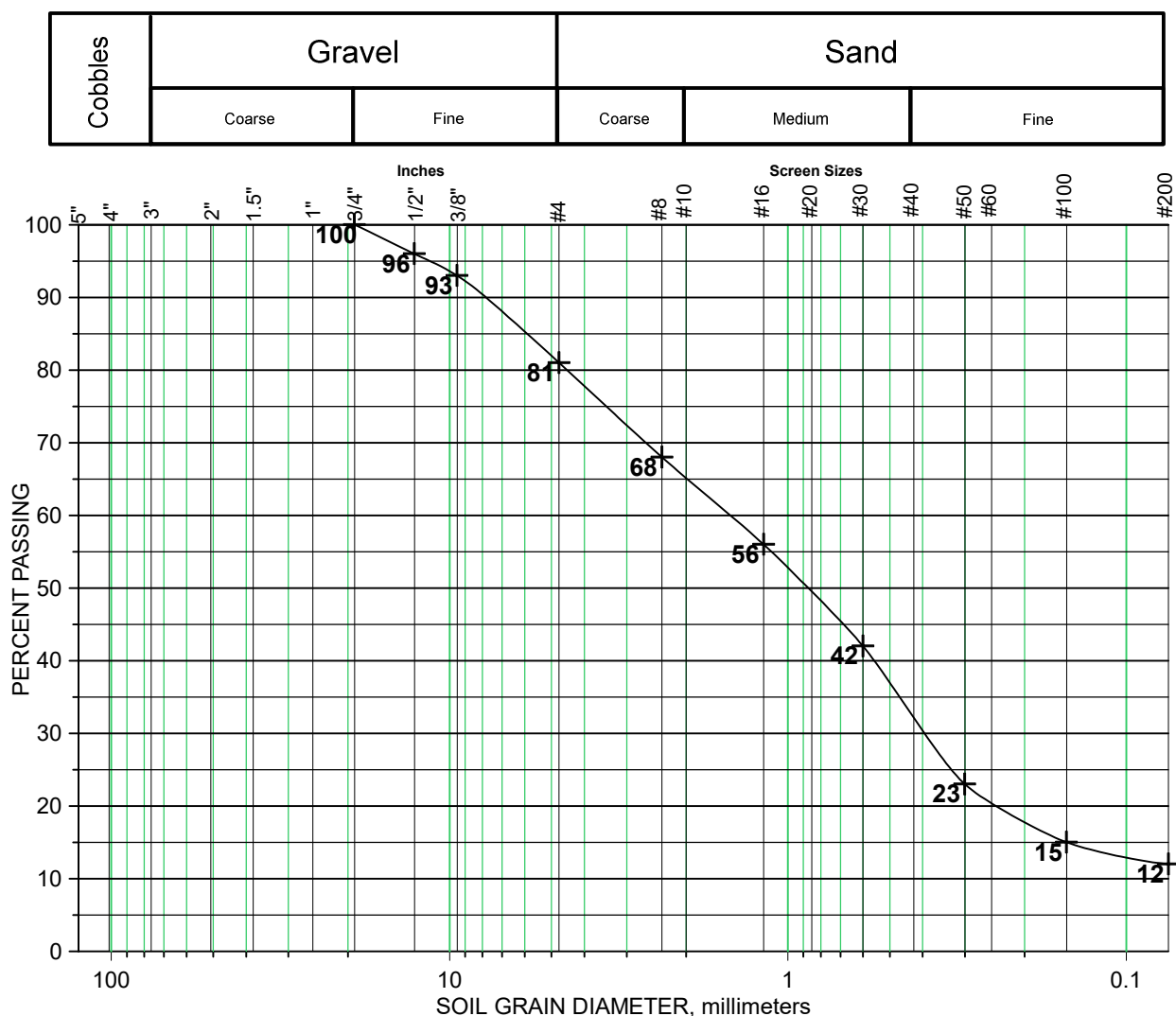
Boise
8653 West Hackamore Drive
Boise, ID 83709
Phone: 208.376.8200

Client
WSP USA
1032 S Silverstone Way
Meridian, ID 83642

Project
BO26280T
US0057480.0861
Elmore County PS2

SAMPLE INFORMATION

SAMPLE NO.:	73698	SAMPLE DATE:	08/10/2026
SPECIMEN SIZE:	Gallon bag	TEST DATE:	08/20/2026
USCS CLASSIFICATION:	Poorly Graded Sand with Silt and Gravel (SP/SM)	SAMPLED BY:	L. Arsenith / J. Nagle
		TESTED BY:	B. Walker
METHOD:	Bulk	BORING NO.:	BH-03
		DEPTH (FT):	5.5 - 7.0



Keith Wildman
Reviewed by Keith Wildman
Laboratory Services Coordinator

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ASTM D6913-17

Report Date: 08/20/2026

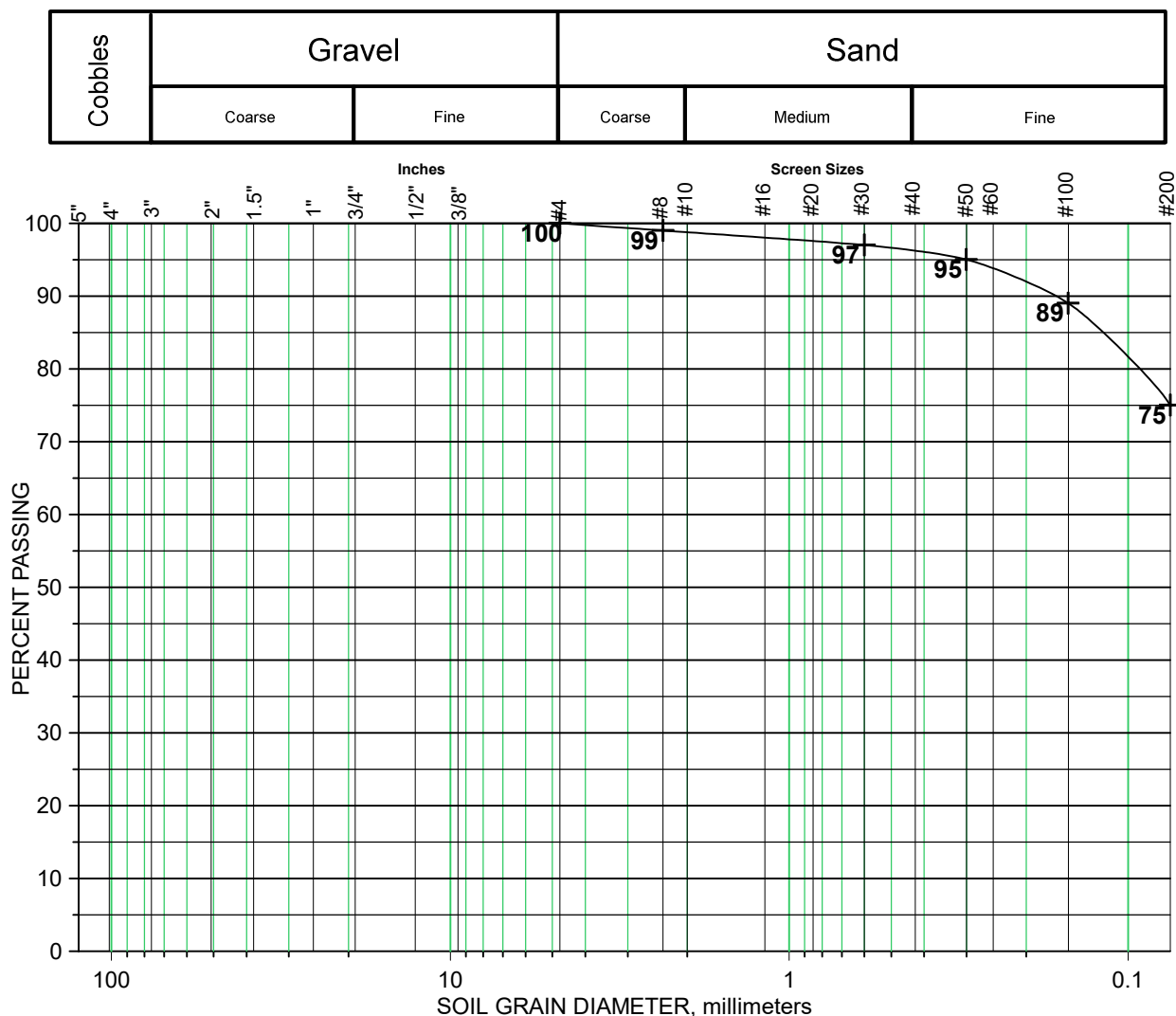
Boise
8653 West Hackamore Drive
Boise, ID 83709
Phone: 208.376.8200

Client
WSP USA
1032 S Silverstone Way
Meridian, ID 83642

Project
BO26280T
US0057480.0861
Elmore County PS2

SAMPLE INFORMATION

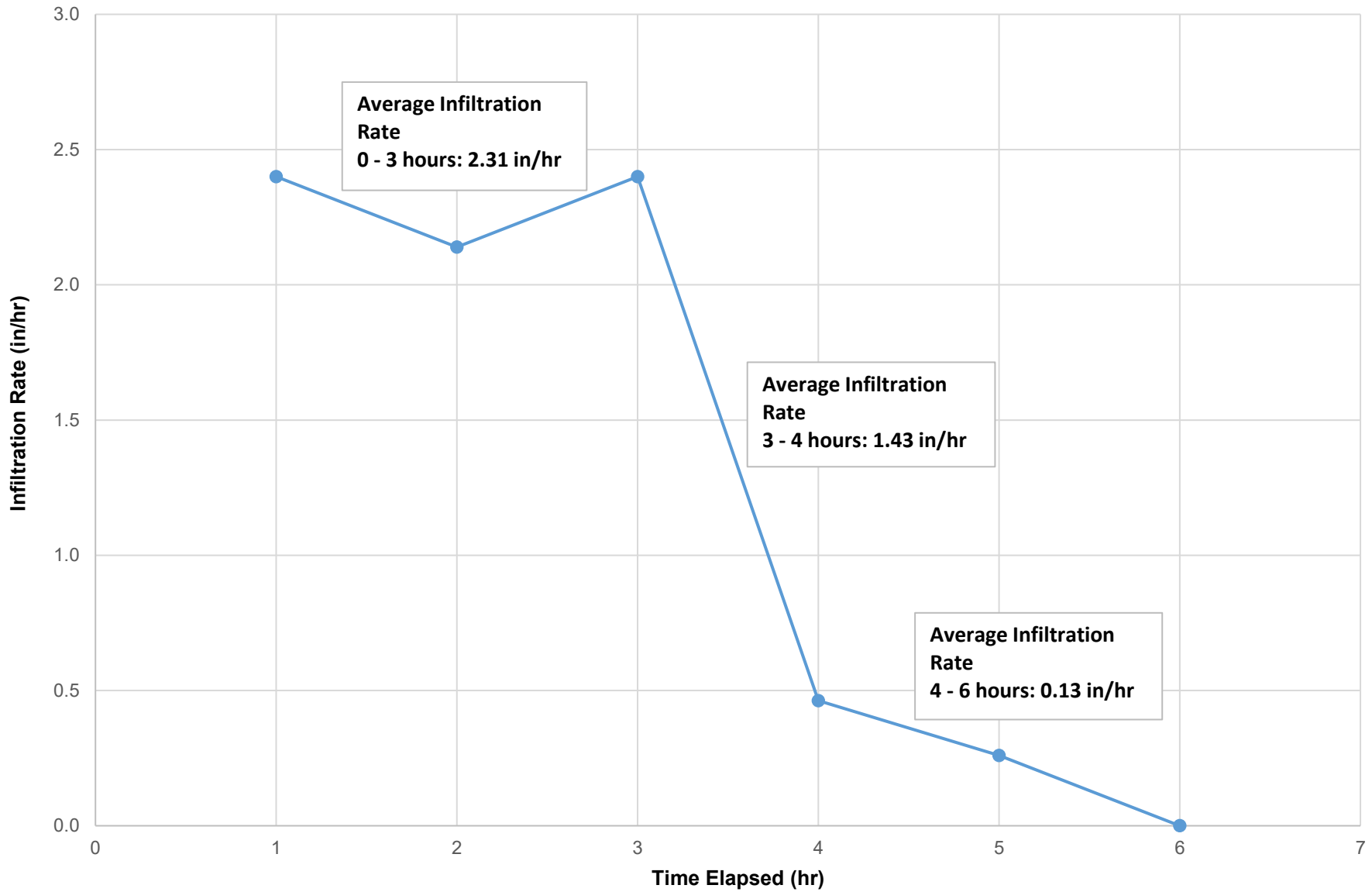
SAMPLE NO.:	73699	SAMPLE DATE:	08/10/2026
SPECIMEN SIZE:	Gallon bag	TEST DATE:	08/20/2026
USCS CLASSIFICATION:	Silt with Sand (ML)	SAMPLED BY:	L. Arsenith / J. Nagle
		TESTED BY:	B. Walker
METHOD:	Bulk	BORING NO.:	BH-05
		DEPTH (FT):	5.0 - 6.5



Keith Wildman
Reviewed by Keith Wildman
Laboratory Services Coordinator

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Appendix D Hydraulic Conductivity and Infiltration Results



LEGEND

—●— Infiltration Rate

CLIENT
Elmore County

CONSULTANT



YYYY-MM-DD 08/28/2026

DESIGNED LJA

PREPARED LJA

REVIEWED

APPROVED

PROJECT
ELMORE COUNTY PS2 GEOTECHNICAL INVESTIGATION
ELMORE COUNTY, IDAHO

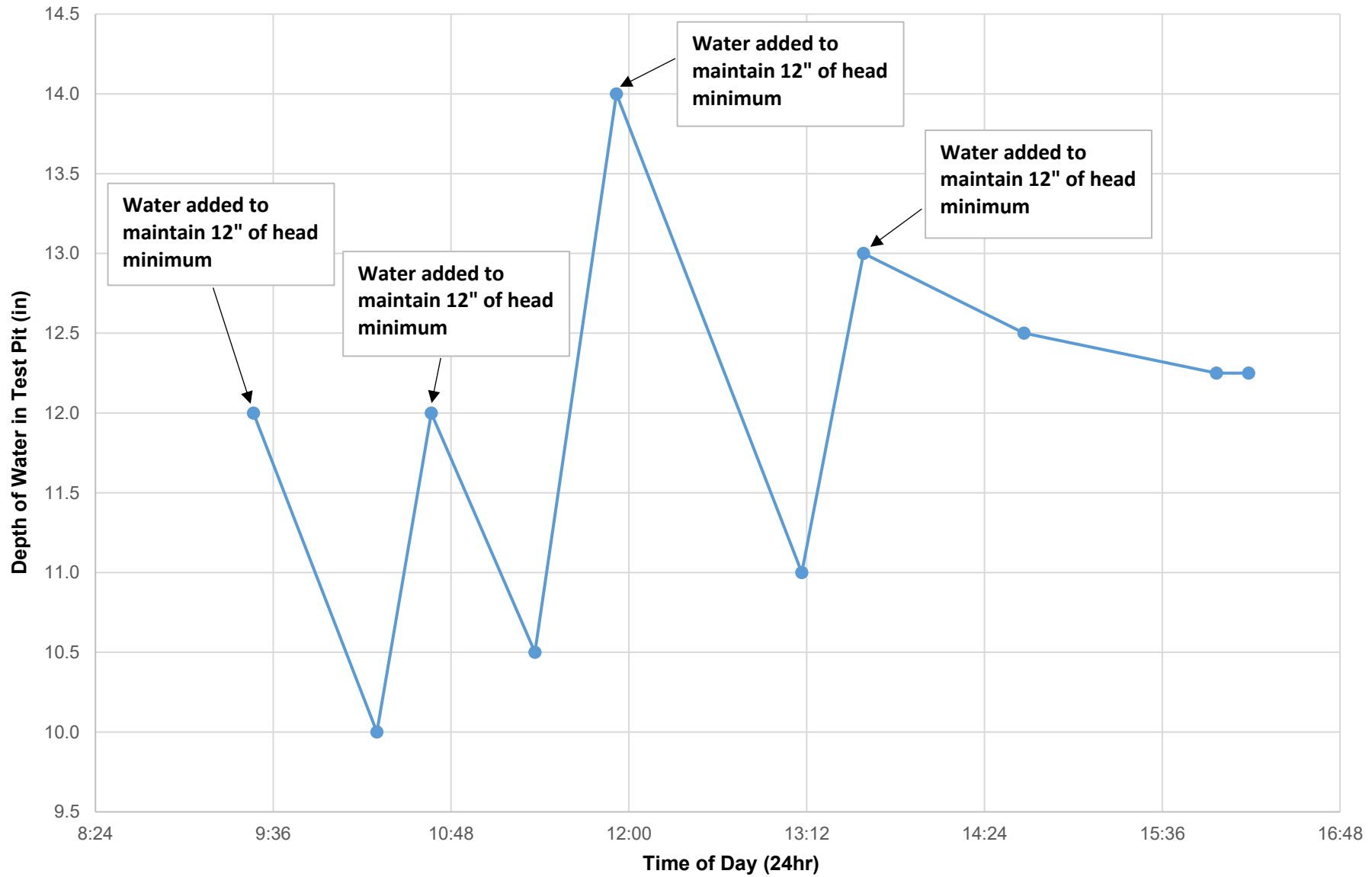
TITLE
**INFILTRATION RATE AND TIME ELAPSED
DURING 6 HOUR PRESOAK PERIOD**

PROJECT NO.
US0057480.0861

PHASE
1

REV.
0

FIGURE
A-1



LEGEND

—●— Depth of Water in Pit

CLIENT
Elmore County

CONSULTANT



YYYY-MM-DD 08/28/2026

DESIGNED LJA

PREPARED LJA

REVIEWED

APPROVED

PROJECT
ELMORE COUNTY PS2 GEOTECHNICAL INVESTIGATION
ELMORE COUNTY, IDAHO

TITLE
**DEPTH OF WATER AND TIME OF DAY
DURING 6 HOUR PRESOAK PERIOD**

PROJECT NO.
US0057480.0861

PHASE
1

REV.
0

FIGURE
A-2

Appendix E Use of This Report

Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled.* No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full.*

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be, and, in general, if you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying it.* A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only*. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may

perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old*.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration*. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists*.



GEOPROFESSIONAL
BUSINESS
ASSOCIATION

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e-mail: info@geoprofessional.org www.geoprofessional.org

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